E-Content

Branch : Mechanical Engineering

Semester : 3rd

Subject : Strength of Materials

Chapters : 1. Stresses and Strains
2. Resilience
3. Moment of Inertia
4. Bending Moment and Shearing Force
5. Bending stresses

Content

2. Resilience

2.1 Strain Energy, Resilience, proof resilience and modulus of resilience

2.2 Strain energy due to direct stresses and Shear Stress

2.3 Stresses due to gradual, sudden and falling load

3. Moment of Inertia

3.1 Concept of moment of inertia and second moment of area

3.2 Radius of gyration

3.3 Theorem of perpendicular axis and parallel axis (with derivation)

3.4 Second moment of area of common geometrical sections : Rectangle, Triangle, Circle (without derivation)

3.5 Second moment of area for L,T and I section, Section modulus

4. Bending Moment and Shearing Force

4.1 Concept of various types of beams and form of loading

4.2 Concept of end supports-Roller, hinged and fixed

4.3 Concept of bending moment and shearing force

4.4 B.M. and S.F. Diagram for cantilever and simply supported beams with and without overhang subjected to concentrated and U.D.L

5. Bending Stresses

5.1 Concept of Bending stresses

5.2 Theory of simple bending, Derivation of Bending Equation

5.3 Use of the equation

5.4 Concept of moment of resistance

5.5 Bending stress diagram

5.6 Section Modulus for rectangular, circular and symmetrical I section

5.7 Calculation of maximum bending stress in beams of rectangular, circular, and T section

1. STRESSES AND STRAIN

INTRODUCTION

Whenever we apply some external force on a body, two situations may arise:

(i) Either body gets deformed
(ii) It resists the force applied on it.

The resistance of the body to the applied force is due to cohesion force acting between the molecules. This resistance offered by the material of the body to the applied force is known as strength of material. The body can oppose the deformation only up to a certain limit known as elastic limit. Within elastic limit, the body regains its original shape and the deformation completely disappears on the removal of external force. Thus, the force of resistance per unit area, offered by the body against deformation if called the stress and deformation per unit length is called strain. The various parts of the machines and structures are designed d on the basis of external forces acting on them.

1.1 BASIC CONCEPT OF LOAD, STRESS AND STRAIN

1.1.1 Load

Load is defined as external force acting upon a machine part. Loads may be classifieds in two ways:
Classification According to the Nature of Application: Following two types of load are important from this point of view.

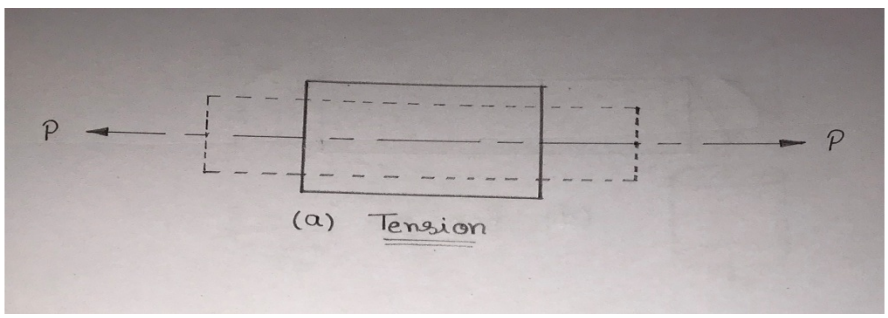
- **Dead Load or Steady Load**: These are the loads which are applied very gradually, i.e. increasing from to their maximum value. These loads do not change their magnitude, direction and point of application .These loads always act vertically. Examples-Load of R.C.C. due to self-weight, load on the bridge girder due to its own weight and weight of the other permanent parts of the bridge structure etc.

- **Live or Fluctuating Loads**: A load is said to be live or fluctuating load, when it changes continuously or suddenly. These can further be classified as-

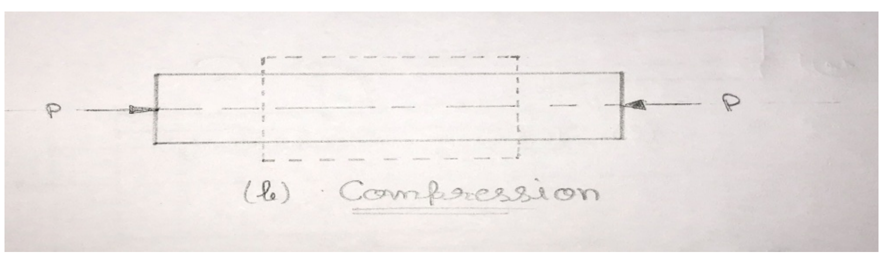
- **Variable load**: A load is said to be a variable load when it changes continuously. The magnitude of such loads varies. Examples-Weight of The traffic according a bridge, snow load on roofs etc.
- **Suddenly applied load or Shock load**: A load is said to be a suddenly applied or shock load, when it is suddenly applied or removed.
- **Impact load**: A load is said to be an impact load, when it is applied with some initial velocity. Examples hammer blow etc.

Classification of Loads According to the Effects Production on the Member:
Following types of load are important from this point of view

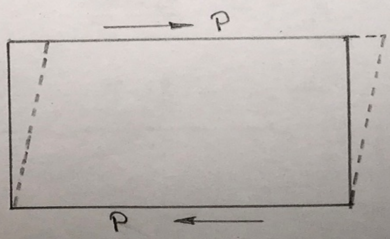
- **Tensile Load**: The loads which tend to pull the member into direction of its application are called tensile loads. These loads cause extension or elongation of the number.



- **Compressive Load**: The load which tends to push together the opposite ends of the member are called compressive loads. These loads cause shortening of the dimension in the direction of their application.

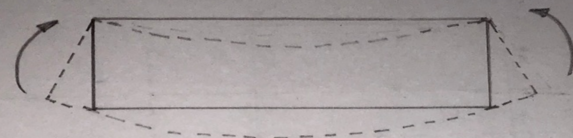


- **Shearing Loads**: The load which tends to cause sliding of one face relative to the other end arecalled Shearing Loads.



(c) Shearing

- **Twisting or Torsional Load**: The load produced by two couples applied at opposite ends of the member, tending to cause one end to rotate above its longitudinal axis relative to other end are called twisting or torsional load.



(d) Bending

- **Bending Loads**: The loads which tend to cause a certain degree of curvature or bending in the member is called bending loads.

(e) Torsion (Twisting)

Effect of Load on A Member

1.1.2 Stress

When a system of external forces or loads acts on a body, a change in its shape and dimension takes place. To oppose the process of deformation, internal resisting forces are set up in the body due to cohesive forces acting between molecules of material. The resisting forces are uniformly distributes over the entire cross-section. The internal resistance per unit area of cross-section is called stress. The difference

between the applied load and stress is that the load is applied externally to the body whereas the stress is induced in the body due to application of load.
If a bar, having uniform cross-section area is acted upon by an external force P, due to cohesion between the molecules, the resistance force is developed in the body against the deformation. If we consider any section XX`, divided by the area of cross-section (A)is caked intensity of stress or stress.

$$\text{i.e., } \textbf{Stress} = \frac{\textbf{Force or load acting on a body (p)}}{\textbf{Cross-sectional area of the body (A)}}$$

In S.I. System unit of stress is N/m^2 or N/mm^2.
In M.K.S. system, the unit of force is kg and the unit of area is m2. Therefore the unit of stress in M.K.S system is kgf/m^2.If the unit of area is cm2 then unit of stress is $kg\backslash cm^2$.

1.1.3 Strain

When an external force or a system of forces is applied on a body, the deformation takes place and there will be change in its dimension .The ratio of this change in the dimensions of the body to the original dimensions is known as strain.

$$\text{i.e. } \textbf{Strain e} = \frac{\textbf{Change in dimensions}(\boldsymbol{\delta l})}{\textbf{Orignal dimensions(l)}} .$$

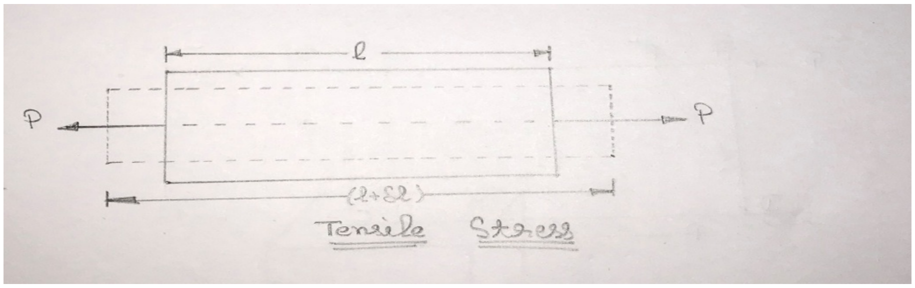
Strain is denoted by e.
It has no unit, because it is the ratio of the same physical quantities. Strain is a measure of the deformation caused due to the original dimensions is known as tensile strain.

1.2 TENSILE, COMPRESSIVE AND SHEAR STRESSES

1.2.1 Tensile Stress

The stress produced in the member when it is subjected to two equal; and opposite pulls, tend to elongate it is called tensile stress. Due to tensile load there will be decrease in cross-sectional area and an increase in length of body.
Let an axial tensile force (p_t) is acting on a member of cross-sectional area, A.

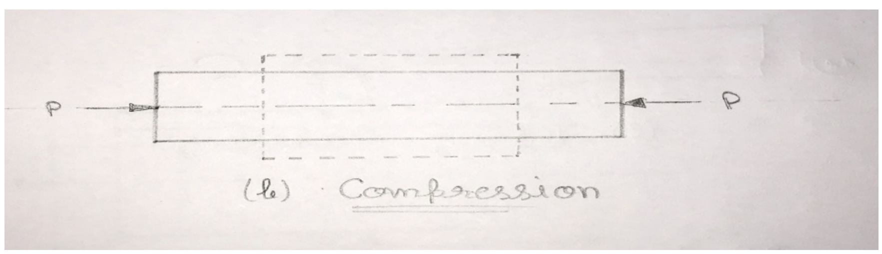
$$\text{Tensile Stress} = \frac{\text{Tensile load}}{\text{Area of cross-section}} \text{ or } F_t = \frac{P_t}{A} .$$



1.2.2 Compressive stress

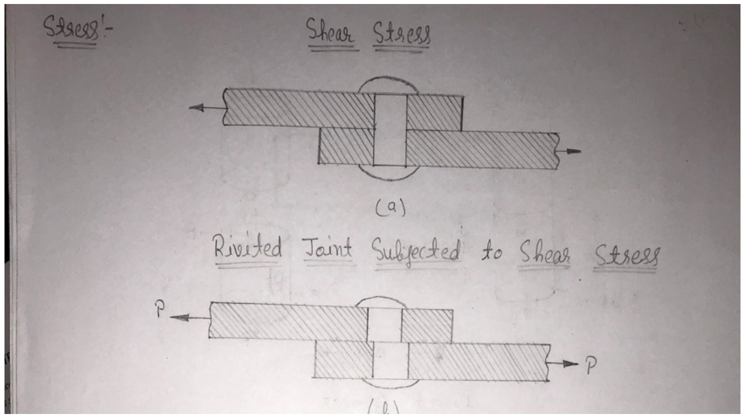
When a compressive force is applied on a body, the stress induced is called compressive stress. When two equal and opposite forces are applied on a body, it gets compressed. There will be an increase in cross-sectional area and a decrease in the length of body. Let an axial compressive load (P_c) is acting on a member of cross-sectional area, A.

$$\text{Compressive Stress} = \frac{\text{Compressive load}}{\text{Area of Cross-section}} \quad \text{or } F_c = \frac{P_c}{A}$$



Shear stress: If two equal and opposite forces are applied in such a way that their line of action is tangential to the resisting section the stress induced is known as shear stress.

$$\text{Shear stress} = \frac{\text{Shear force}}{\text{shear area}}$$



1.3 LINEAR STRAIN, LATERAL STRAIN, SHEAR STRAIN, VOLUMETRIC STRAIN

Linear Strain

Linear strain of a deformed body is defined as the ratio of the change in length of the body due to the deformation to its original length in the direction of the force. If l is the original length and dl the change in length occurred due to the deformation, the linear strain e induced is given by e=dl/l.

Lateral Strain

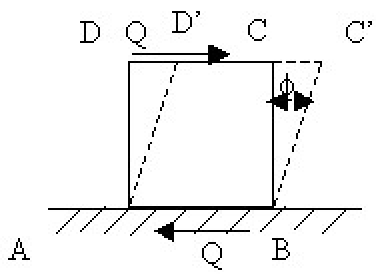
Lateral strain of a deformed body is defined as the ratio of the change in length (breadth of a rectangular bar or diameter of a circular bar) of the body due to the deformation to its original length (breadth of a rectangular bar or diameter of a circular bar) in the direction perpendicular to the force.

Volumetric Strain

Volumetric strain of a deformed body is defined as the ratio of the change in volume of the body to the deformation to its original volume. If V is the original volum and dV the change in volume occurred due to the deformation, the volumetric strain ev induced is given by ev =dV/V

Shear Strain

Shear strain is defined as the strain accompanying a shearing action. It is the angle in radian measure through which the body gets distorted when subjected to an external shearing action. It is denoted by *.



1.4 CONCEPT OF ELASTICITY, ELASTIC LIMIT AND LIMIT OF PROPORTIONALITY

1.4.1 Elasticity

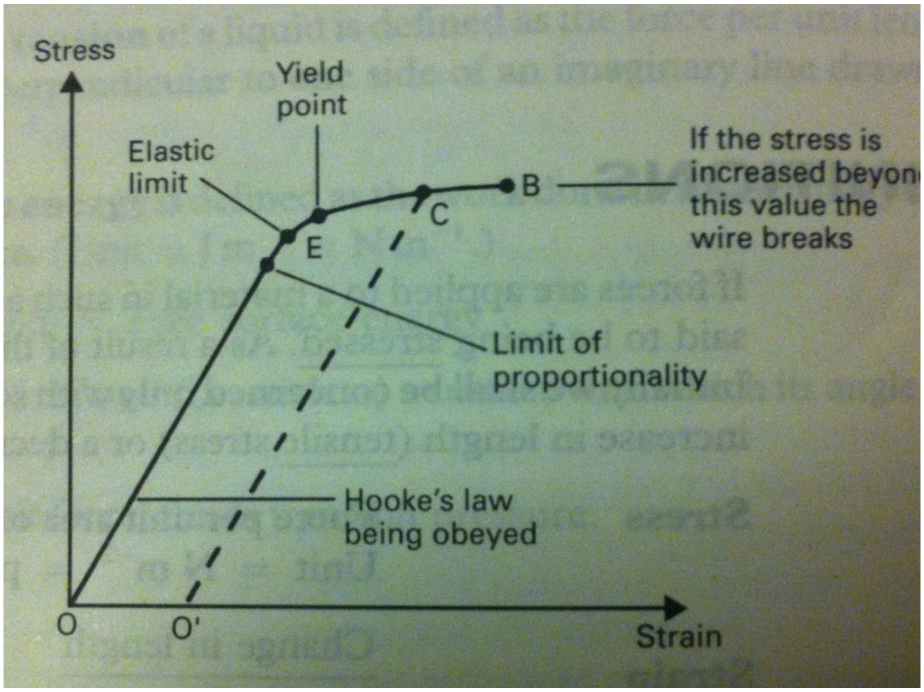
Elasticity is the property of material of a body by virtue of which it opposes any change being produced in its shape or size by external force and its tends to regain its original shape and size after the removal of the external force.

1.4.2 Elastic limit

A material is said to be perfectly elastic if the deformation produced by external forces completely disappears on the removal of external forces. For most brittle materials, stresses beyond the elastic limit result in fracture with almost no plastic deformation.

1.4.3 Limit of Proportionality

In the diagram, these are straight line from point O to A, which represent that directly stress is proportional to strain. Beyond point A, the curve slightly deviates from the straight line. Hook's law holds good upto point A and it is known as limit f proportionality. Limit of proportionality may be defined as that stress at which the stress-strain curve begins to deviate from the straight line. The limit of proportionality is the point beyond which Hooke's law is no longer true when stretching a material. The elastic limit is the point beyond which the material you are stretching becomes permanently stretched so that the material does not return to its original length when the force is removed.



1.5 HOOK'S LAW AND ELASTIC CONSTANTS

1.5.1 Hook's Law

Robert Hooke (1635-1703) experimentally established in 1676, that when a material is loaded within elastic limit, the stress is directly proportional to the strain produced by the stress.

i.e. Stress α Strain

$$\sigma \ \alpha \ \varepsilon$$

$$\sigma = E\,\varepsilon$$

where E is constant called as Coefficient of elasticity or Elastic constant.

Elastic Constants

Following are the elastic constants

- **Modules of Elasticity or Young's Modules:** it may be defined as the ratio of tensile stress and tensile strain or ratio of compressive stress and compressive strain. It is denoted by E. Young's modules as the same units as that of stress, i.e. N/mm^2

$$\text{Young's modules, } E = \frac{\text{Tensile stress}}{\text{Tensile strain}}$$

Sr.No.	Material	Modules of Elasticity (E) in GPa
1.	Steel	200 to 220
2.	Wrought iron	190 to to 200
3.	Cast iron	100 to 16
4.	Copper	90 to 110
5.	Brass	80 to 90
6.	Aluminum	60 to 80
7.	Timber	10

- **Modules of Rigidity or Shear Modules:** The ratio of shear stress and shear strain is known as modules of rigidity or shear modulus. This is denoted by G or C.

$$\text{Modules of rigidity, } G = \frac{\text{Shear stress}(\tau)}{\text{Shear strain}(\phi)}$$

S.No.	Material	Modulus of rigidity (G) in Gpa
1.	Steel	80 to 100
2.	Wrought	80to 90
3.	iron	40 to 50
4.	Cast iron	30 to 50
5.	Copper	30 to 50
6.	Brass	10

- **Bulk Module:** When a body is subjected to three mutually perpendicular normal stresses of equal intensity, the ratio of normal stress to the corresponding volumetric strain is known as bulk modulus. It is denote by K. The unit of bulk modules is $N/mm^{2.}$

$$\mathbf{K = \frac{Normmal\ stress(\sigma)}{Volumetric\ strain(\epsilon_v)}}$$

1.6 STRESS-STRAIN CURVE FOR DUCTILE AND BRITTLE MATERIALS

To plot stress and strain diagram, nominal stresses are calculated by dividing the loads by the original cross-section and the corresponding the stress and strain are plotted on the graph paper by taking strain along X-axis and stresses along Y-axis.

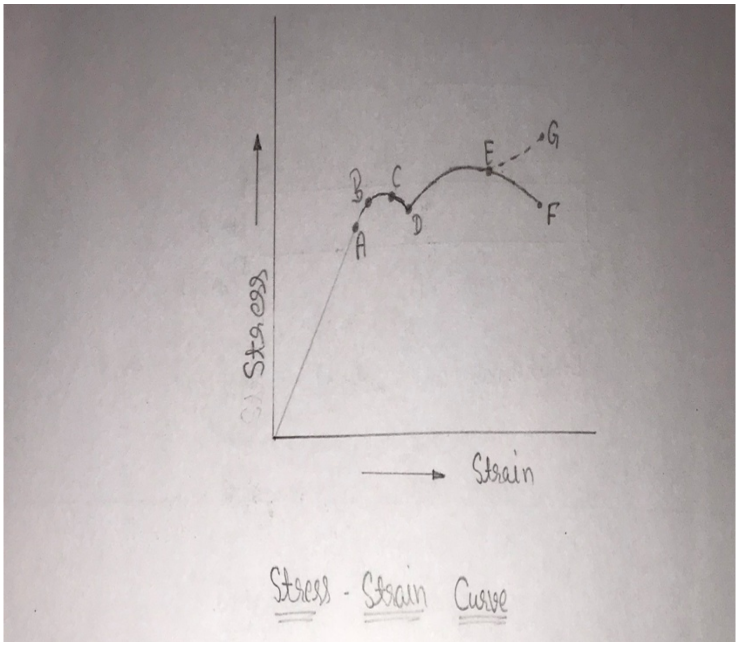


Figure shows that in stress-strain diagram for ductile materials, the curve starts from the origin, showing that there are no initial stresses or strain in the test specimen. The ductile specimen passes through the following stages during loading:-

- Limit of proportionality (Point A)
- Elastic limit (Point B)
- Yield point (Upper yield point C and lower yield point D)
- Maximum or ultimate load point (Point F)
- Breaking point (Point G)

- **Limit of proportionality:** Starting from origin to point A, Hooke's law is obeyed i.e. the stress is proportion to straight. Thus the limit up to which the stress is directly proportional to strain is called limit of proportionality. Therefore OA is straight line. The stress corresponding to point A is called proportionality. Therefore OA is straight line. The stress corresponding to point A is called proportion limit stress (f_p).

$$\text{Limit of proportionality} = \frac{\text{Load at elastic limit}}{\text{Original cross-sectional area}}$$

- **Elastic Limit:** The portion of the diagram between AB is not straight line, but up to point B, the material remains elastic, i.e n removal of load, no permanent deformation is observed and the material will regain its original shape and size.

$$\textbf{Elastic limit} = \frac{\textbf{Load at elastic limit}}{\textbf{Original area of cross-section}}$$

- **Yield Point:** Beyond the point B, the material goes to the plastic stage until the upper yield point C is reached i.e. the removal of load does not allow returning the specimen to its original form. Point C is upper yield point.

- **Ultimate Stress Point:** From point E onwards, the strain hardening phenomenon becomes predominant. The stress again starts increasing up to point F because the material picks up the ability to resist increasing stress, nut the elongation now increases at much faster rate. Point F it is the maximum stress to which the material can be subjected. T point a material begins and the cross-sectional area starts decreasing at a rapid rate.

- **Breaking Point:** Beyond the point F, elongation will continue at gradually decreasing lesser load and ultimately the breaks at point G. Point G is known as ‘breaking point’ or fracture point.

1.7 NOMINAL STRESSES

Nominal stress is defined as the force on the object divided by the original area. it will be cleared if we compare the above definition with true stress definition(which is "the force on the object divided by the actual area".

1.8 YIELD POINT, PLASTIC STAGE

1.8.1 Yield point

Beyond the point B, the material goes to the plastic stage until the upper yield point C is reached i.e. the removal of load does not allow to return the specimen to its original form. Point C is upper yield point & Point D is lower yield point. At this point the cross-sectional area of the material point

1.8.2 Plastic Stage

A permanent deformation or change in shape of solid body without fracture under the action of sustained force small changes in the density of crystals due to plastic deformation.

1.9 ULTIMATE STRESS AND BREAKING STRESS

1.9.1 Ultimate Stress

Maximum or ultimate stress of a material is defined as the ratio of the maximum load which a specimen is subjected in a tensile test and the original cross-sectional area of the specimen

$$\text{Ultimate stress} = \frac{\text{Maximum load}}{\text{Original cross-secytional area}}$$

1.9.2 Breaking Stress

It is defined as the ratio of the maximum load at which fracture occurs in a specimen subjected to a tensile test and the original cross-sectional area of the specimen.

1.10 PERCENTAGE ELONGATION

If L_0 is the original gauge length and L_f is the final length of the specimen, then

$$\text{Percentage elongation} = \frac{Lf\text{-}L0}{L0} * 100$$

1.11 PROOF STRESS AND WORKING STRESS

- **Proof Stress**

Proof stress is the stress necessary to cause a permanent extension equal to a defined percentage of gauge length.

- **Working Stress**

In actual practice, the material is not subjected upto ultimate stress, but only upto fraction of ultimate stress. This stress is known as working stress is also known as allowable stressor permissible stress.

$$\text{Working stress} = \frac{\text{Ultimate strss}}{\text{Factor of safety}}$$

1.12 FACTOR OF SAFETY

The ratio of ultimate stress and working stress is called factor of safety. It is also known as factor of ignorance.

$$\text{Factor of Safety} = \frac{\text{Ultimate stress}}{\text{Working stress}}$$

1.13 POISSON'S RATIO

It is the ratio of the proportional decrease in a lateral measurement to the proportional increase in length in a sample of material that is elastically stretched. It is denoted by (μ) or (1/m).

Poisson's Ratio, $(\frac{1}{m}) = \frac{lateral\ strain}{longotudnal\ \text{strain}}$

1.14 THERMAL STRESS AND STRAIN

When the temperature of a body is changed, the change in dimensions takes place. If the member is free to expand or contract, no stresses will be induced. But, if the change in length is prevented, stress is developed in the body. This stress is called thermal stress or temperature stress.

If the length of the bar of uniform section is l, the temperature of body changes from t1 to t2 α is the coefficient of linear expansion.

The extensions due to rise in temperature = α (t2 – t1).

If the ends of body are fixed to prevent extensions produced due to rise in temperature, the temperature strain = $\frac{\alpha(t2 - t1)l}{l} = \alpha(t2 - t1)$

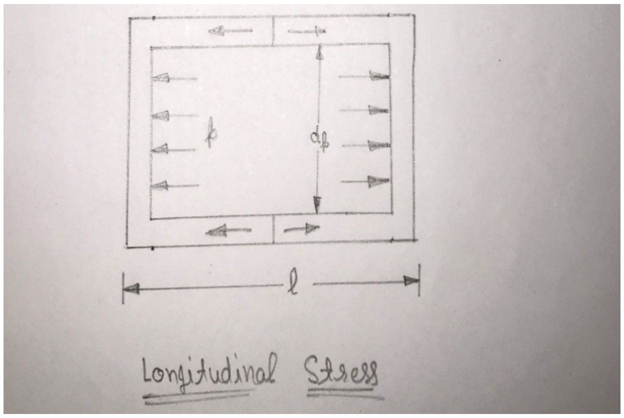
1.15 LONGITUDINAL AND CIRCUMFERENTIAL STRESSES IN SEAM LESS THIN WALLED CYLINDRICAL SHELLS

- **Longitudinal Stresses**

The stresses which act along the length of the cylinder are called longitudinal or axial stress. Consider a thin cylinder subjected to an internal pressure, which tends to split up in to two pieces.

Let, P is the pressure, f_l is the longitudinal stress induced in the cylinder.

Diameter of shell is d and the thickness is 't'.



Bursting force = P x Area on which P acts = P x $(\frac{\pi}{4} d^2)$

Resisting force = Stress xArea on which stress acts = $f_l x(\pi d.t)$

For equilibrium, the bursting force should be equal to resisting force.

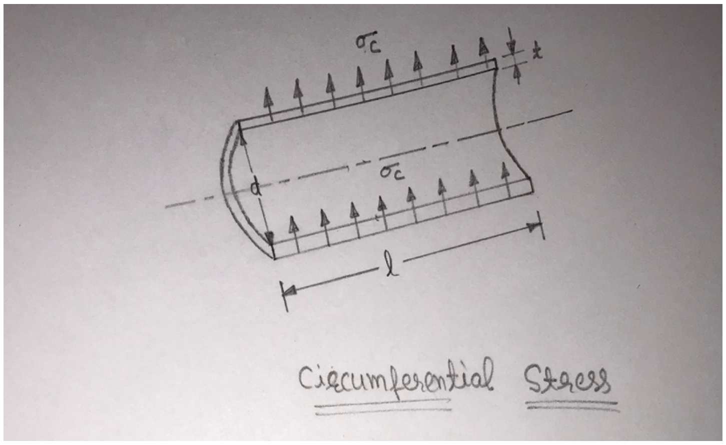
i.e. Bursting = Resisting force

or $P\frac{\pi}{4} d^2 = f_{l.} \pi X t$

$f_l = \frac{Pd}{4t}$

- **Circumferential Stress or Hoop Stress**

  The stresses which act along the circumference of the cylinder are called circumferential or hoop stresses. These are also known as tangential stresses. Due to hoop stresses, the cylinder may split up into two troughs. Consider a thin cylinder shell of internal diameter, d and length, l. and t is the thickness of cylindrical shell.



Let the hoop stresses develop due to the internal fluid pressure. The intensity of internal pressure is denoted by P. Due to the internal pressure, the shell may split up into two troughs along XX axis.

Let the hoop stresses developed due to internal fluid pressure is denoted by f_h.

Total bursting force in shell = Intensity of pressure x Area on which P acts

=P. (d x l)

Resisting force = f_h x Area on with f_h is acting

For equilibrium, bursting force must be equal to resisting force.

Bursting force = Resisting force

P. d. l= f_hx 2. l. t

$$F_h = \frac{Pd}{2t}$$

1.16 INTRODUCTION TO PRINCIPAL STRESSES

Principal stress is the maximum normal stress a body can have at its some point. It represents purely normal stress. If at some point principal stress is said to have acted it does not have any shear stress component. Principal stresses are maximum and minimum value of normal stresses on a plane (when rotated through an angle) on which there is no shear stress. Principal plane is that plane on which the principal stresses act and shear stress is zero.

$$\boxed{\sigma_{1,2} = \frac{\sigma_x + \sigma_y}{2} \pm \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau_{xy}^2}}$$

2. RESILIENCE

2.1 STRAIN ENERGY, RESILIENCE, PROOF RESILIENCE AND MODULUS OF RESILIENCE

- **Strain Energy**
  The amount of work done in straining the body with in elastic limit is called as Strain energy. Strain energy is same as work done. It is denoted by ‘U’.

- **Resilience**
  When an elastic body is subjected to a force it may undergo a linear deformation. The force acts on the body throughout this deformation process. Hence work is done by the force for the deformation. This work done by the force is stored in the material as internal energy (strain energy). This stored energy is used to restore the original shape of the body on removal of force. The energy stored when a body is strained within the elastic limit is known as resilience.

- **Proof Resilience**
  The strain energy stored in the material will be maximum when it is strained up to the elastic limit. This maximum strain energy stored in the material when it is strained up to the elastic limit is known as Proof resilience.

- **Modulus of Resilience**
  The Proof resilience per unit volume is known as modulus of resilience. Hence it is the maximum strain energy that can be stored in an elastic material per unit volume.

2.2 STRAIN ENERGY DUE TO DIRECT STRESSES AND SHEAR STRESS

2.6 STRAIN ENERGY STORED IN A BODY DUE TO SHEAR STRESS

Let consider a cubical block ABCD of length l whose one face AD is fixed while shear force P is applied on the opposite face BC. Let τ is the shear stress induced and ϕ is the corresponding shear strain. Let G is the modulus of rigidity.

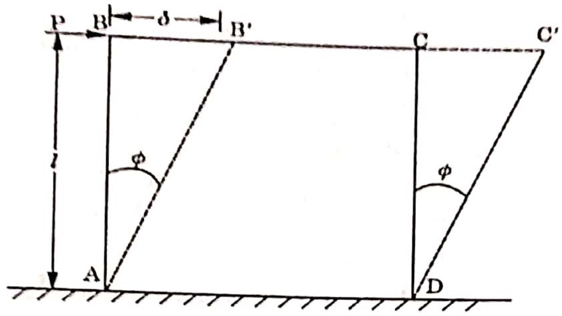


Fig. 2.3 : Strain Energy Stored Due to Shear Stress

As the load P is applied gradually,

$$\therefore \quad \text{Average load} = \frac{0+P}{2} = \frac{P}{2}$$

Strain energy stored, U = Workdone in deforming the rectangular block

= Average load × Deformation

$$= \frac{P}{2} \times CC'$$

$$= \frac{P}{2} \times DC.\phi \qquad (\because CC' = DC.\phi)$$

$$= \frac{1}{2}\tau \times (BC \times l) \times DC \times \phi \qquad (\because P = \tau \times BC \times l)$$

$$= \frac{1}{2}\tau \times \phi\,(BC \times DC \times l)$$

$$= \frac{1}{2} \times \text{Stress} \times \text{Strain} \times \text{Volume}$$

$$= \frac{1}{2}\tau \times \frac{\tau}{C} \times V$$

Where V = Volume of cubical block.

$$\therefore \quad U = \frac{\tau^2}{2C} \times V$$

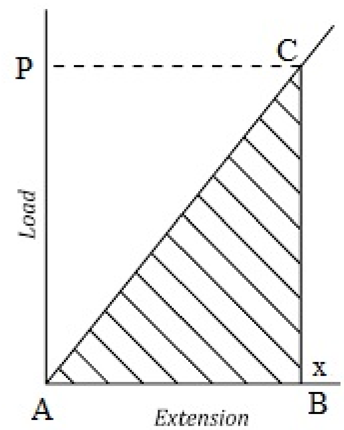
Modulus of resilience = Strain energy stored per unit volume

$$= \frac{\tau^2}{2C}$$

2.3 STRESSES DUE TO GRADUAL, SUDDEN AND FALLING LOAD

- **Strain Energy due to Gradual applied Load**

  Let us consider a body which is subjected with tensile load which is increasing gradually up to its elastic limit from value 0 to value P and therefore deformation or extension of the body is also increasing from 0 to x and we can see it in following load extension diagram as displayed here.



σ= Stress developed in the body

E = Young's Modulus of elasticity of the material of the body

A= Cross sectional area of the body

P = Gradually applied load which is increasing gradually up to its elastic limit from value 0 to value P

P = σ. A

x = Deformation or extension of the body which is also increasing from 0 to x

L = Length of the body

V= Volume of the body = L.A

U = Strain energy stored in the body

As we have already discussed that when a body will be loaded within its elastic limit, the work done by the load in deforming the body will be equal to the strain energy stored in the body.

Strain energy stored in the body = Work done by the load in deforming the body

Strain energy stored in the body = Area of the load extension curve

Strain energy stored in the body = Area of the triangle ABC

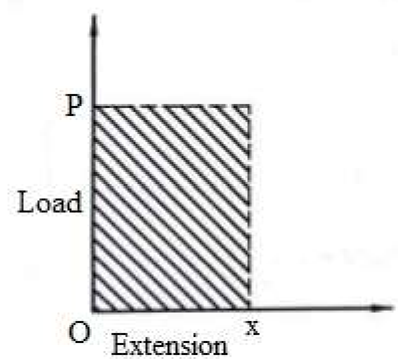
U = (1/2) . AB . BC
U = (1/2) x. P
Let us use the value of P = σ. A, which is determined above
U = (1/2) x. σ. A

As, x = σ. L/ E
Therefore strain energy stored in a body, when load will be applied gradually, will be given by following equation.

$$\frac{\sigma^2}{2E} \text{ x } V$$

- **Strain Energy due to Suddenly applied Load**

  Let us see the load extension diagram as displayed here for this case where body will be subjected with sudden load and we will find out here the stress induced in the body due to sudden applied load and simultaneously we will also secure the expression for strain energy for this situation.



σ = Stress developed in the body due to sudden applied load
E = Young's Modulus of elasticity of the material of the body
A= Cross sectional area of the body
P = Sudden applied load which will be constant throughout the deformation process of the body
x = Deformation or extension of the body
L = Length of the body
V= Volume of the body = L.A

U = Strain energy stored in the body

Strain energy stored in the body = Work done by the load in deforming the body
Strain energy stored in the body = Area of the load extension curve
Strain energy stored in the body = P. x
U = P. x
As we know that maximum strain energy stored in the body U will be provided by the following expression as mentioned here.

$$U = \frac{\sigma^2}{2E} \times V$$

$$U = \frac{\sigma^2}{2E} \times A.\ L$$

$$P.\ x = \frac{\sigma^2}{2E} \times A.\ L$$

$$P \times \frac{\sigma}{E} \times L = \frac{\sigma^2}{2E} \times A.\ L$$

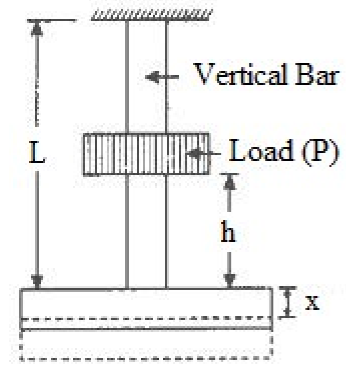
$$P = \sigma \times A / 2$$

$$\sigma = 2P/A$$

Therefore, we can say here that maximum stress induced in the body due to sudden applied load will be twice the stress induced in the body with same value of load applied gradually.

- **Strain Energy due to impact applied Load**

  Let us see the following figure, where we can see one vertical bar which is fixed at the upper end and there is collar at the lower end of the bar. Let us think that one load is being dropped over the collar of the vertical bar from a height of h as displayed in following figure



Let us go ahead step by step for easy understanding, however if there is any issue we can discuss it in comment box which is provided below this post.
We have following information from above figure where a body is subjected with an impact load.

σ= Stress developed in the body due to impact load
E = Young's Modulus of elasticity of the material of the body
A= Cross sectional area of the body
P = Impact load
x = Deformation or extension of the body i.e. vertical bar
L = Length of the body i.e. vertical bar
V= Volume of the body i.e. vertical bar = L.A
U = Strain energy stored in the body i.e. vertical bar

Strain energy stored in the vertical bar = Work done by the load in deforming the vertical bar
Strain energy stored in the vertical bar = Load x Displacement
Strain energy stored in the vertical bar = P. (h + x)
U = P. (h + x)
As we know that strain energy stored in the body U will be provided by the following expression as mentioned here.

$$U = \frac{\sigma^2}{2E} \times V$$

$$U = \frac{\sigma^2}{2E} \times A.\ L$$

Therefore, we will have

$$P.\ (h + x) = \frac{\sigma^2}{2E} \times A.\ L$$

Let use the value of the extension or deformation "x" in above equation and we will have.

$$P\left(h + \frac{\sigma\ L}{E}\right) = \frac{1}{2} \cdot \frac{\sigma^2}{E}(A \times L)$$

$$Ph + \frac{P.\sigma\ L}{E} = \frac{\sigma^2}{2E}(AL)$$

$$\frac{\sigma^2}{2E} - \frac{P.\ \sigma}{A.E} = \frac{P.h}{A.L}$$

$$\sigma^2 - \frac{2P\sigma}{A} = \frac{2PhE}{A.L}$$

$$\sigma^2 - \frac{2P\sigma}{A} + \frac{P^2}{A^2} = \frac{2PhE}{A.L} + \frac{P^2}{A^2}$$

$$\left(\sigma - \frac{P}{A}\right)^2 = \frac{P^2}{A^2} + \frac{2.P.h.E}{A.L}$$

$$\left(\sigma - \frac{P}{A}\right) = \sqrt{\frac{P^2}{A^2} + \frac{2P.h.E}{A.L}}$$

$$\sigma = \frac{P}{A} + \sqrt{\frac{P^2}{A^2} + \frac{2P.h.E}{A.L}}$$

3. MOMENT OF INERTIA

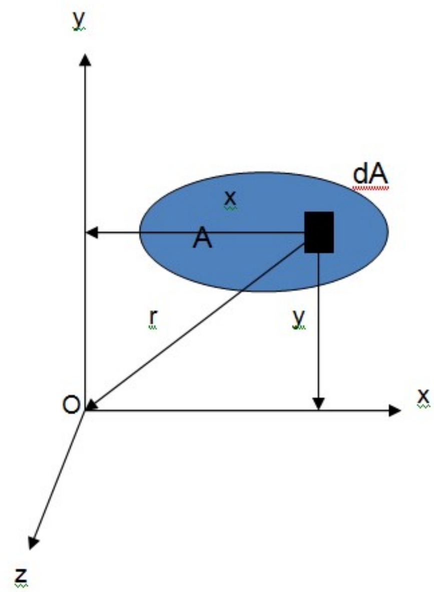
3.1 CONCEPT OF MOMENT OF INERTIA AND SECOND MOMENT OF AREA

- **Moment Of Inertia**
  The product of the mass and the square of the distance of the center of gravity from an axis are known as moment of the inertia. The moment of inertia is represented by "I".
  The value of the moment of inertia is always positive, regardless of location of the axis. Units are such as $kgmm^2$ or kgm^2.
  Moments of inertia for the entire area A about the x and y axes are

$$I_x = \sum y^2 dm \text{ and } I_y = \sum x^2 dm.$$

- **Second Moment of Area**
  The product of the area and the square of the distance of the centeroid from an axis are known as second moment of the area. The second area moment of is represented by "I".
  Units are length to the 4th power, such as mm^4 or m^4.
  Moments of inertia for the entire area A about the x and y axes are

$$I_x = \sum y^2 dA \text{ and } I_y = \sum x^2 dA.$$



3.2 RADIUS OF GYRATION

Radius of gyration of a body about an axis of rotation is defined as the radial distance of a point from the axis of rotation at which, if whole mass of the body is assumed to be concentrated, its moment of inertia about the given axis would be the same as with its actual distribution of mass.

Mathematically, the radius of gyration is the root mean square distance of the object's parts from either its center of mass or a given axis, depending on the relevant application. It is actually the perpendicular distance from point mass to the axis of rotation.

Consider area A with moment of inertia I_x. Imagine that the area is concentrated in a thin strip parallel to the x axis with equivalent I_x.

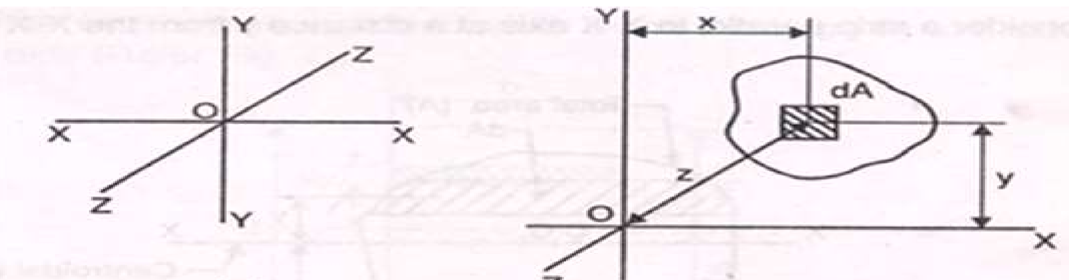
$$I_x = k_x^2 A \qquad k_x = \sqrt{\frac{I_x}{A}}$$

k_x = Radius of gyration with respect to the x axis

3.3 THEOREM OF PERPENDICULAR AXIS AND PARALLEL AXIS (WITH DERIVATION)

- **Theorem of Perpendicular Axis**

  The theorem states that the M.O.I of any figure about the axis perpendicular to mutually perpendicular axis is equal to sum of M.O.I about these two axes of the same figure.



(a) (b)

Proof. Consider an elementary area dA, at a distance x from Y-Y axis and y from X-X axis and z from Z-Z axis. Z-Z axis is an axis perpendicular to X-X and Y-Y as shown in the Fig. 2.

By Pythagoras theorem,

$$z^2 = x^2 + y^2$$

Moment of inertia of an elementary area about Z-Z axis

$$= z^2 \cdot dA = (x^2 + y^2) \cdot dA = x^2 \cdot dA + y^2 \cdot dA$$

Total moment of inertia of the whole section about Z-Z axis,

$$I_{ZZ} = \Sigma x^2 \cdot dA + \Sigma y^2 \cdot dA$$

But $\Sigma x^2 \cdot dA = I_{YY}$

and, $\Sigma y^2 \cdot dA = I_{XX}$

$$\therefore \quad I_{ZZ} = I_{XX} + I_{YY}$$

Z-Z axis is called **polar axis** and I_{ZZ} is known as **polar moment of inertia.** Polar moment of inertia is useful in analysing the torsional stresses. Polar moment of inertia is also denoted by I_P.

- **Theorem of Parallel Axis**

  The parallel axis theorem states that the M.O.I of an area about any axis is equal to the M.O.I of the area about its own centroid, plus the product of the area & the square of the distance between the centroid of the area & the axis about which M.O.I is required

  Mathematically, it is given by

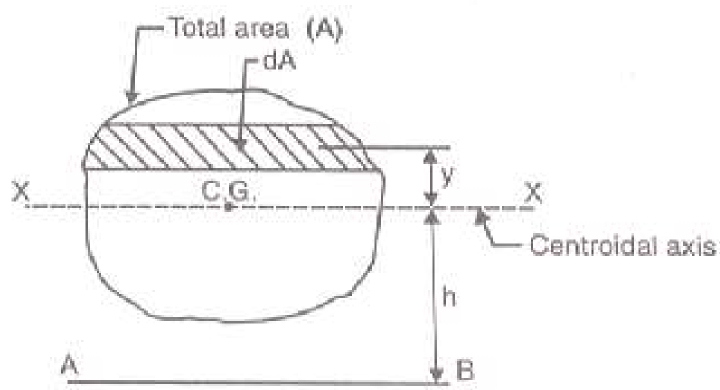
$$I_{AB} = I_G + Ah^2$$

where I_{AB} = Moment of inertia of the given area about AB

I_G = Moment of inertia of the given area about its own centroid

A = Area of the section

h = Distance between the centroid of the section and the axis AB.



Let the area of strip = dA

Moment of inertia of area dA about X-X axis = dA . y^2

$\therefore$ Moment of inertia of the total area about X-X axis, I_{XX} or $I_G = \Sigma \, dA \, . \, y^2$

Moment of inertia of the area dA about AB

$$= dA \, . \, (h + y)^2 = dA \, . \, (h^2 + y^2 + 2hy)$$

$\therefore$ Moment of inertia of the total area about AB,

$$I_{AB} = \Sigma \, dA \, . \, (h^2 + y^2 + 2hy)$$

$$= \Sigma \, dA \, . \, h^2 + \Sigma \, dA \, . \, y^2 + \Sigma \, dA \, . \, 2hy$$

or $\quad I_{AB} = h^2 \, \Sigma \, dA + \Sigma \, dA \, . \, y^2 + 2h \Sigma \, dA \, . \, y \quad (\because h \text{ or } h^2 \text{ is constant})$

or $\quad I_{AB} = h^2 \, A + I_G + 2h \Sigma \, dA \, . \, y \quad$ .....(i)

$(\because \Sigma \, dA = A \text{ and } \Sigma \, dA \, . \, y^2 = I_G)$

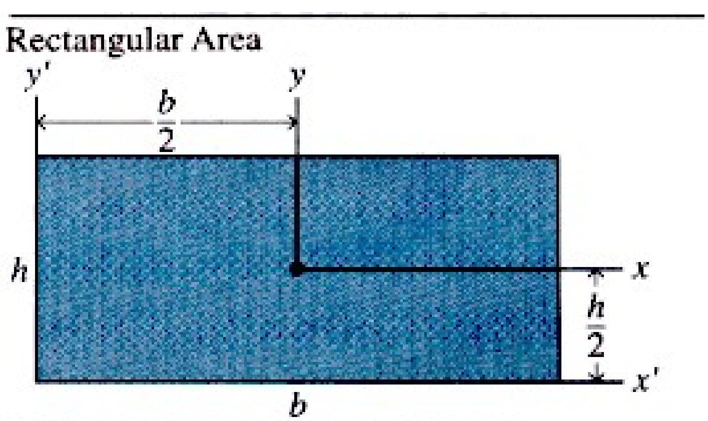
$\Sigma \, dA$. y represents the moment of the total area about X-X axis. As the distance of the centroid of the total area from X-X is zero, hence $\Sigma \, dA$. y will be equal to zero.

Substituting $\Sigma \, dA \, . \, y = 0$ in equation (i), we get

$$I_{AB} = Ah^2 + I_G + 0$$

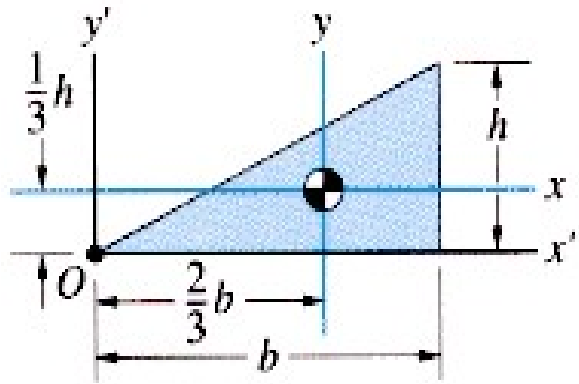
or $\quad I_{AB} = I_G + Ah^2$

3.4 SECOND MOMENT OF AREA OF COMMON GEOMETRICAL SECTIONS RECTANGLE, TRIANGLE, CIRCLE (WITHOUT DERIVATION)



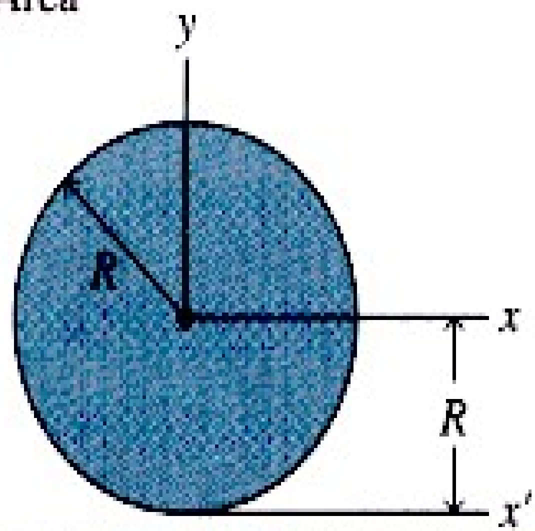
$A = bh$	$I_x = \dfrac{bh^3}{12}$	$I_{x'} = \dfrac{bh^3}{3}$
	$I_y = \dfrac{hb^3}{12}$	$I_{y'} = \dfrac{hb^3}{3}$
	$I_{xy} = 0$	$I_{x'y'} = \dfrac{b^2h^2}{4}$

Triangular Area



$A = \dfrac{1}{2}bh$	$I_x = \dfrac{bh^3}{36}$	$I_{x'} = \dfrac{bh^3}{12}$
	$I_y = \dfrac{hb^3}{36}$	$I_{y'} = \dfrac{hb^3}{4}$
	$I_{xy} = \dfrac{b^2h^2}{72}$	$I_{x'y'} = \dfrac{b^2h^2}{8}$

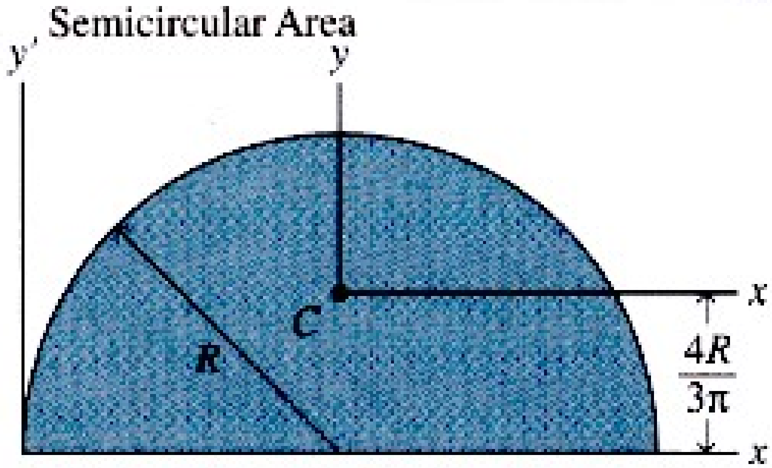
Circular Area



$$A = \pi R^2 \qquad I_x = \frac{\pi R^4}{4} \qquad I_{x'} = \frac{5\pi R^4}{4}$$

$$I_y = \frac{\pi R^4}{4}$$

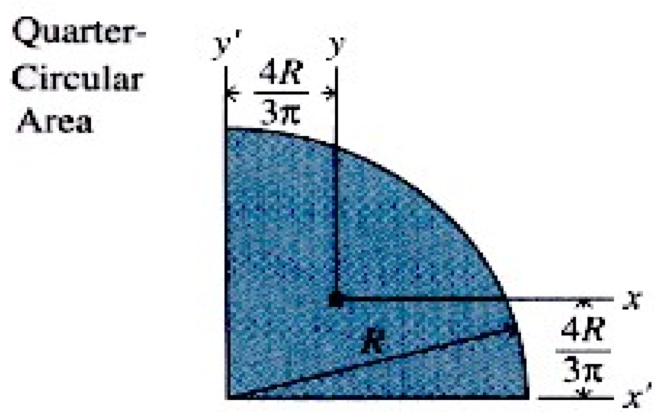
$$I_{xy} = 0$$



$$A = \frac{1}{2}\pi R^2 \qquad I_x = \frac{\pi R^4}{8} - \frac{8R^4}{9\pi} \qquad I_{x'} = \frac{\pi R^4}{8}$$

$$I_y = \frac{\pi R^4}{8}$$

$$I_{xy} = 0 \qquad I_{x'y'} = \frac{2R^4}{3}$$

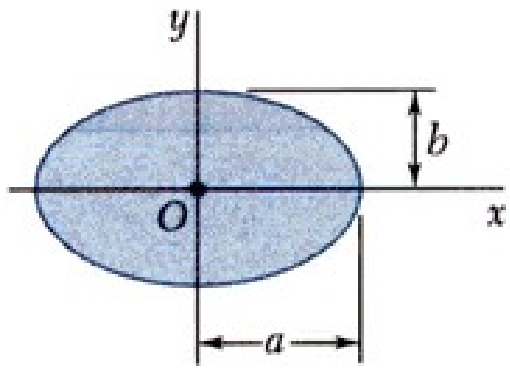


$$A = \frac{1}{4}\pi R^2 \qquad I_x = \frac{\pi R^4}{16} - \frac{4R^4}{9\pi} \qquad I_{x'} = \frac{\pi R^4}{16}$$

$$I_{y'} = \frac{\pi R^4}{16}$$

$$I_{xy} = \frac{(9\pi - 32)R^4}{72\pi} \qquad I_{x'y'} = \frac{R^4}{8}$$

Ellipse

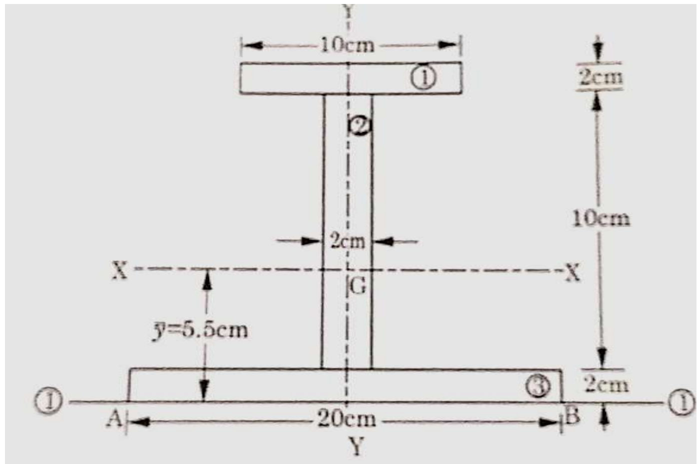


$$\overline{I}_x = \frac{1}{4}\pi a b^3$$

$$\overline{I}_y = \frac{1}{4}\pi a^3 b$$

3.5 SECOND MOMENT OF AREA FOR L,T AND I SECTION, SECTION MODULUS

- **Second Moment Of Area For L,T And I**



Component	Area (A) (cm^2)	Centroidal Distance y from Axis 1-1 (cm)	Ay (cm^3)	Ay^2 (cm^4)	I_{self} (cm^4)
1	20	13	260	3380	$\frac{10\times 2^3}{12} = 6.67$
2	20	7	140	980	$\frac{2\times (10)^3}{12} = 166.67$
3	40	1	40	40	$\frac{20\times 2^3}{12} = 13.33$
	$\Sigma A = 80$		$\Sigma Ay = 440$	$\Sigma Ay^2 = 4400$	$\Sigma I_{self} = 186.67$

Distance of centroidal axis XX from axis 1-1,

$$\bar{y} = \frac{\Sigma Ay}{\Sigma A} = \frac{440}{80} = 5.5 \text{ cm}$$

$$I_{1\text{-}1} = \Sigma I_{self} + \Sigma Ay^2$$

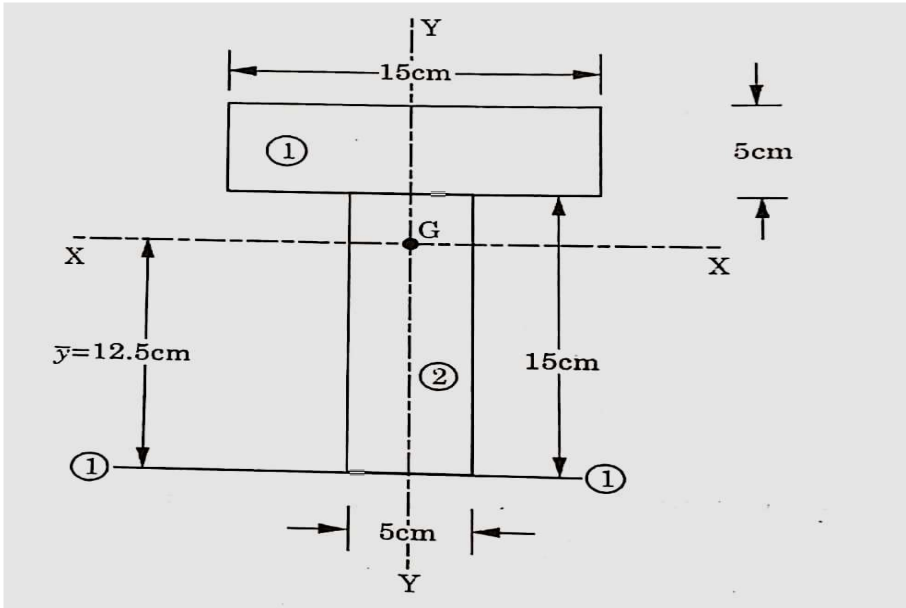
$$= 186.67 + 4400 = 4586.67 \text{ cm}^4$$

But $$I_{1\text{-}1} = I_{XX} + (\Sigma A).\ \bar{y}^2$$

$$4586.67 = I_{XX} + 80 \times (5.5)^2$$

or $$I_{XX} = 2166.67 \text{ cm}^4 \quad \textbf{Ans.}$$

$$I_{YY} = \frac{2\times(10)^3}{12} + \frac{10\times 2^3}{12} + \frac{2\times(20)^3}{12} = 1506.67 \text{ cm}^4 \quad \textbf{Ans.}$$


Y
15cm
①
5cm
G
X
X
$\bar{y}$=12.5cm
②
15cm
①
①
5cm
Y

Solution. Let us split this T-section into two components- Component (1) and component (2). As this section is symmetrical about YY-axis, therefore, the centroid will lie on this axis.

Component	Area (A) (cm^2)	Centroidal Distance y from Axis 1-1 (cm)	Ay (cm^3)	Ay^2 (cm^4)	I_{self} (cm^4)
1	75	17.5	1312.5	22968.75	$\frac{15\times(5)^3}{12}=156.25$
2	75	7.5	562.5	4218.75	$\frac{5\times(15)^3}{12}=1406.25$
	$\Sigma A = 150$		$\Sigma Ay = 1875$	$\Sigma Ay^2 = 27187.5$	$\Sigma I_{self} = 1562.5$

Distance of centroidal axis from axis 1-1,

$$\bar{y} = \frac{\Sigma Ay}{\Sigma A}$$

$$= \frac{1875}{150} = 12.5 \text{ cm}$$

$$I_{1\text{-}1} = \Sigma I_{self} + \Sigma Ay^2$$

$$= 1562.5 + 27187.5$$

$$= 28750 \text{ cm}^4$$

But

$$I_{1\text{-}1} = I_{XX} + (\Sigma A).\ \bar{y}^2$$

$$28750 = I_{XX} + 150 \times (12.5)^2$$

or

$$I_{XX} = 5312.5 \text{ cm}^4 \quad \textbf{Ans.}$$

$$I_{YY} = \frac{5\times(15)^3}{12} + \frac{15\times(5)^3}{12}$$

$$= 1562.5 \text{ cm}^4 \quad \textbf{Ans.}$$

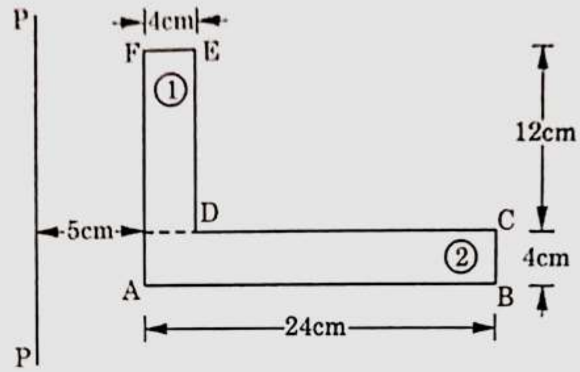


Fig. 3.16

Solution. Let us divide the given section into two rectangles : (1) and (2) and calculate the moment of inertia of each about axis P–P with the help of theorem of parallel axis.

M.O.I. of rectangle (1) about P–P axis,

$$I_{PP1} = \frac{12 \times 4^3}{12} + (12 \times 4)(5+2)^2$$

$$= 64 + 48 \times 49 = 2416 \text{ cm}^4$$

M.O.I. of rectangle (2) about P–P axis,

$$I_{PP_2} = \frac{4 \times 24^3}{12} + (4 \times 24)(5+12)^2$$

$$= 4608 + 96 \times 289 = 32352 \text{ cm}^4$$

Hence M.O.I of given section about P-P axis,

$$I_{PP} = I_{PP_1} + I_{PP_2}$$

$$= 2416 + 32352 = 34768 \text{ cm}^4 \quad \textbf{Ans.}$$

- **Section Modulus**

  It is defined as the ratio of M.O.I of a section about its neutral axis to the distance of the outermost layer or extreme edge from the neutral axis. It is denoted by "Z".

$$\boldsymbol{Z = \frac{I}{y}}$$

Where I = M.O.I about centrodial axis (neutral axis).

y = Distance of the outer most layer from centrodial axis.

4. BENDING MOMENT AND SHEARING FORCE

4.1 CONCEPT OF VARIOUS TYPES OF BEAMS AND FORM OF LOADING

➢ **Beam**

A beam is a structural member used for bearing loads. It is typically used for resisting vertical loads, shear forces and bending moments.

➢ **Types of Beam**

- simply supported beam
- Cantilever beam
- Overhanging beam
- Continuous beam
- Fixed beam

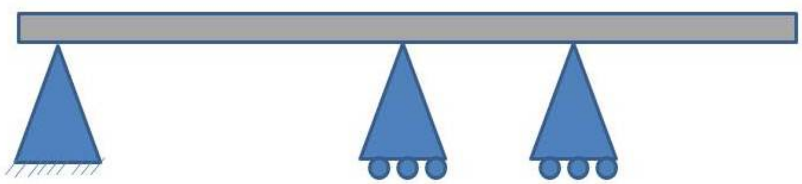
▪ **Simply Supported Beam**: A simply supported beam is a type of beam that has pinned support at one end and roller support at the other end. Depending on the load applied, it undergoes shearing and bending. It is the one of the simplest structural elements in existence.

Simply Supported Beam

▪ **Cantilever Beam:** Cantilever beams a structure member of which one end is fixed and other is free. This is one of the famous type of beam use in trusses, bridges and other structure member. This beam carry load over the span which undergoes both shear stress and bending moment.

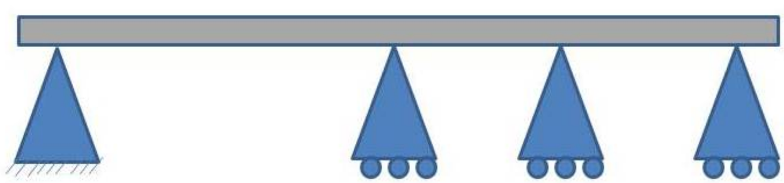
Cantilever Beam

- **Overhanging Beam:** Overhanging beam is combination of simply supported beam and cantilever beam. One or both of end overhang of this beam. This beam is supported by roller support between two ends. This type of beam has heritage properties of cantilever and simply supported beam.



Overhanging Beam

- **Continuous Beam:** This beam is similar to simply supported beam except more than two support are used on it. One end of it is supported by hinged support and other one is roller support. One or more supports are use between these beams. It is used in long concrete bridges where length of bridge is too large.



Continuous Beam

- **Fixed Beam:** This beam is fixed from both ends. It does not allow vertical movement and rotation of the beam. It is only under shear stress and no moment produces in this beams. It is used in trusses, and other structure.

- **Types of Loading**

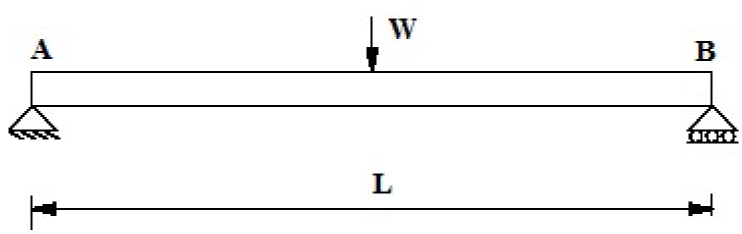
A beam is usually horizontal member and load which will be acting over the beam will be usually vertical loads. There are following types of loads as mentioned here and we will discuss each type of load in detail.

- Point load or concentrated load
- Uniformly distributed load
- Uniformly varying load

- **Point load or concentrated load:**

Point load or concentrated load, as name suggest, acts at a point on the beam. If we will see practically, point load or concentrated load also distributed over a small area but we can consider such type of loading as point loading and hence such type of load could be considered as point load or concentrated load.

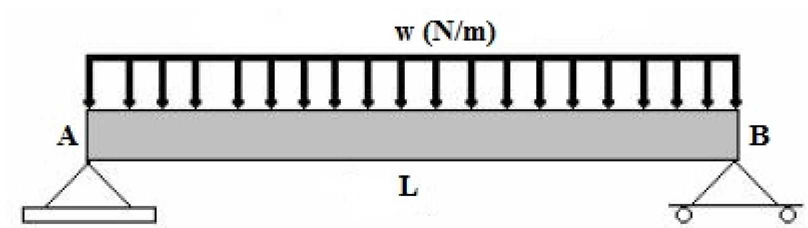
Following figure displayed here indicates the beam AB of length L which will be loaded with point load W at the midpoint of the beam. Load W will be considered here as the point load.



- **Uniformly distributed load:**

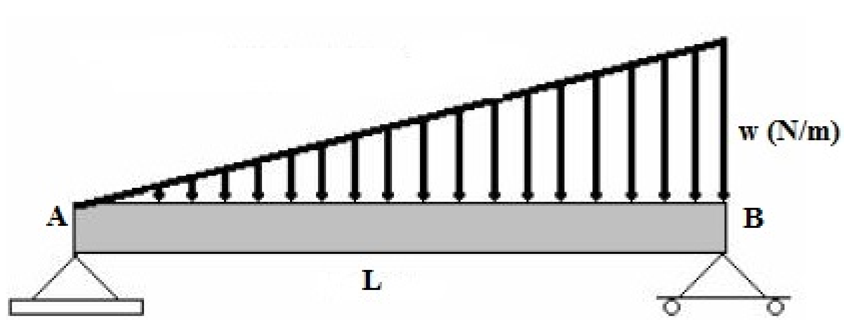
Uniformly distributed load is the load which will be distributed over the length of the beam in such a way that rate of loading will be uniform throughout the distribution length of the beam.

Uniformly distributed load is also expressed as U.D.L and with value as w N/m. During determination of the total load, total uniformly distributed load will be converted in to point load by multiplying the rate of loading i.e. w (N/m) with the span of load distribution i.e. L and will be acting over the midpoint of the length of the uniformly load distribution.
Let us consider the following figure, a beam AB of length L is loaded with uniformly distributed load and rate of loading is w (N/m).
Total uniformly distributed load, P = w*L



- **Uniformly varying load:**

Uniformly varying load is the load which will be distributed over the length of the beam in such a way that rate of loading will not be uniform but also vary from point to point throughout the distribution length of the beam. Uniformly varying load is also termed as triangular load. Let us see the following figure, a beam AB of length L is loaded with uniformly varying load.
We can see from figure that load is zero at one end and increases uniformly to the other end. During determination of the total load, we will determine the area of the triangle and the result i.e. area of the triangle will be total load and this total load will be assumed to act at the C.G of the triangle.
Total load, P = w*L/2



4.2 CONCEPT OF END SUPPORTS-ROLLER, HINGED AND FIXED

- **Beam**

  A beam is a structural member used for bearing loads. It is typically used for resisting vertical loads, shear forces and bending moments.

- **Types of End Support**

- **Roller supports**

  Roller support allows thermal expansion and contraction of the span and prevents damage on other structural members such as a pinned support. The typical application of Roller supports is in large bridges. In civil engineering, roller supports can be seen at one end of a bridge.

  Roller support cannot prevent translational movements in horizontal or lateral directions and any rotational movement but prevents vertical translations. Its reaction force is a single linear force perpendicular to, and away from, the surface (upward or downward). This support type is assumed to be capable of resisting normal displacement.

- **Pinned support**

  Pinned support attaches the only web of a beam to a girder called a shear connection. The support can exert a force on a member acting in any direction and prevent translational movements, or relative displacement of the member-ends in all directions but cannot prevent any rotational movements. Its reaction forces are single linear forces of unknown direction or horizontal and vertical forces which are components of the single force of unknown direction.[5]

Pinned support is just like a human elbow. It can be extended and flexed (rotation), but you cannot move your forearm left to right (translation). One benefit of pinned supports is not having internal moment forces and only their axial force playing a big role in designing them.

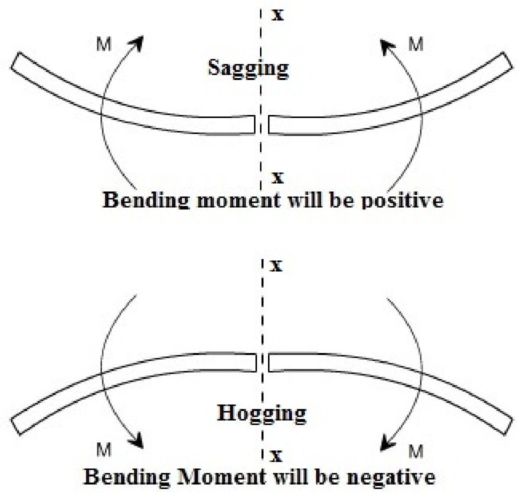
- **Fixed support**

Rigid or fixed supports maintain the angular relationship between the joined elements and provide both force and moment resistance. It exerts forces acting in any direction and prevents all translational movements (horizontal and vertical) as well as all rotational movement of a member. These supports' reaction forces are horizontal and vertical components of a linear resultant; a moment.[5] It is a rigid type of support or connection. The application of the fixed support is beneficial when we can only use single support, and people most widely used this type as the only support for a cantilever. They are common in beam-to-column connections of moment-resisting steel frames and beam, column and slab connections in concrete frames.

4.3 CONCEPT OF BENDING MOMENT AND SHEARING FORCE

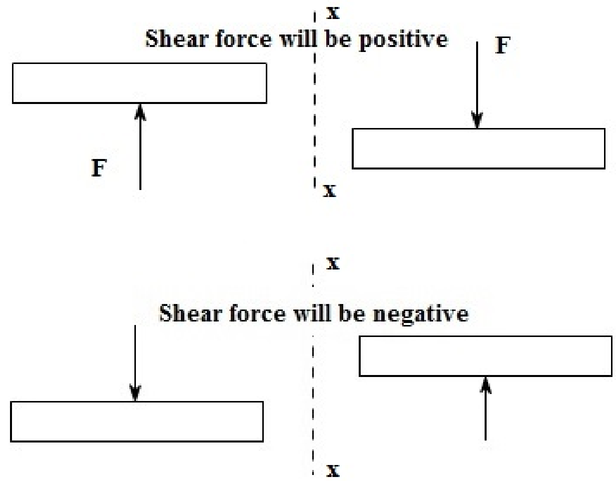
- **Bending Moment**

It may be defined as the algebraic sum of the moments of all vertical forces either to the left or to the right of a section. A B.M. causing concavity upwards will be taken as negative and called as sagging B.M. Similarly, a B.M. causing convexity upwards will be taken as positive and called hogging. Bending moment diagram i.e. BMD will tell you the variation of bending moment throughout the length of the beam.



- **Shear Force**

  It may be defined as the algebraic sum of all vertical forces either to the left or to the right hand side of a section. Shear force having an upward direction to the right hand side of a section or downwards to the left of the section will be taken positive. Similarly, a negative S.F. will be one that has a downward direction to the right of the section or upward direction to the left of the section. Shear force diagram i.e. SFD will tell you the variation of shear force along the length of the beam.



4.4 B.M. AND S.F. DIAGRAM FOR CANTILEVER AND SIMPLY SUPPORTED BEAMS WITH AND WITHOUT OVERHANG SUBJECTED TO CONCENTRATED AND U.D.L

- **Simply Supported Beam : U.D.L. over the whole span**
  A beam, Simply Supported Beam : U.D.L. over the whole span is a structural element that primarily resists loads applied laterally to the beam's axis. Its mode of deflection is primarily by bending.

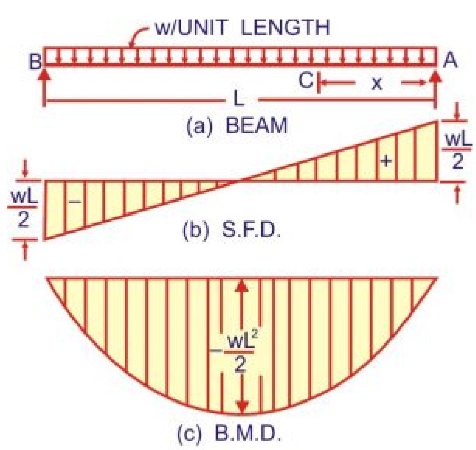


Fig. 3.18.

The reaction $R_A = R_B = \frac{wL}{2}$ (due to symmetry)

S.F.D. : $F_x = +R_A - wx = \frac{wL}{2} - wx$ ...... (linear)

At x=0, $F_A = +\frac{wL}{2}$; At $x = \frac{L}{2}$, $F = 0$

At x=L, $F_B = +\frac{wL}{2} - wL = -\frac{wL}{2}$

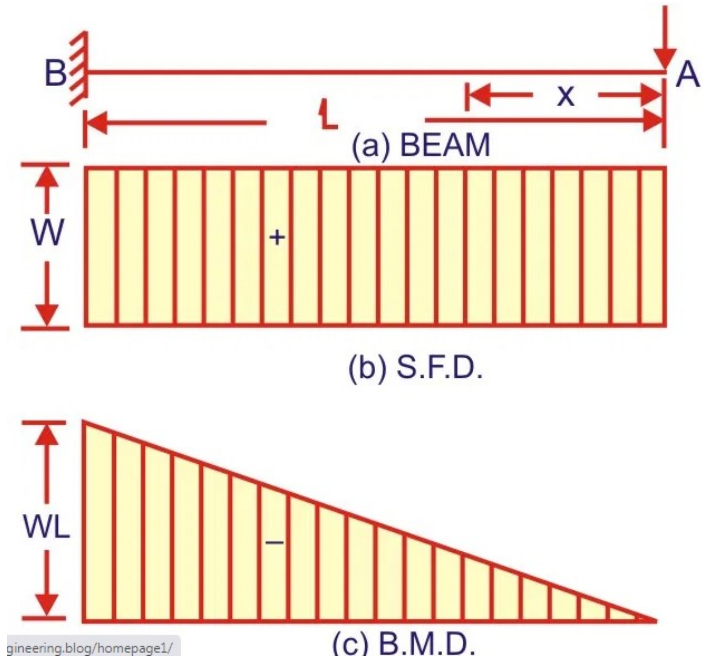
B.M.D. : $M_x = -R_A.x + wx.\frac{x}{2} = -\frac{wL}{2}x + \frac{wx^2}{2}$ .... (parabolic)

At x=0, M_A=0 ; At x=L, M_B=0

At $x = \frac{L}{2}$, $M = -\frac{wL}{2} \cdot \frac{L}{2} + \frac{w}{2}\left(\frac{L}{2}\right)^2 = -\frac{wL^2}{8}$

- **Bending moment and shear force diagram of a cantilever beam**

Cantilever : Point Load at the End



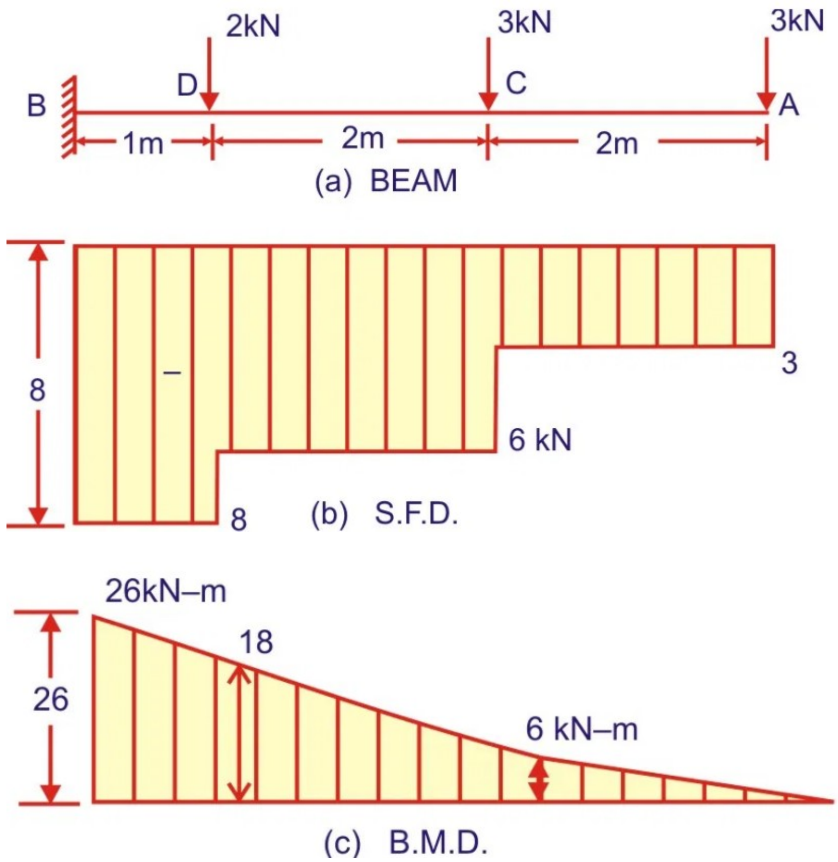
At section x from the end A, Fx = – W1 and is constant for any position of the section. The S.F.D. will, therefore, be rectangle of height W. Bending moment at a section x from end A is given by:

Mx = + W, x ………(straight line)

At x=0, MA=D ; x=1, MB=WL.

The B.M.D. will thus be a triangle having zero ordinate at A and WL at B.

Cantilever : Several Point Loads



S.F.D. : Between A and C,

Fx= – 3 kN (constant)

Between C and D,

F_x = -3-3=-6 kN (constant)

Between D and B,

F_x = -3-3-2=-8 kN (constant)

The S.F.D. will, therefore, consist of several rectangles having different ordinates,

B.M.D. : Between A and C,

$M_x = 3x$ ….. (linear)
When x=0, MA=0. When x=2m, MC=3×2=6 kN.m
Between C and D.

$M_x = 3x+3(x-2)$ ……(linear)

When x=2, MC=6 kN.m (as before)

When x=4, MD=(3×4)+(3×2)=18 kN.m

Between D and B,

$M_x = 3x+3(x-2)+2(x-4)$ …… (linear)

When x=4, MD=12+6=18 kN.m (as before)

When x=5, MB=15+9+2=26 kN.m.

5. BENDING STRESSES

5.1 CONCEPT OF BENDING STRESSES

Bending stress is the normal stress that is induced at a point in a body subjected to loads that cause it to bend. When a load is applied perpendicular to the length of a beam (with two supports on each end), bending moments are induced in the beam.

Normal Stress

A normal stress is a stress that occurs when a member is loaded by an axial force. The value of the normal force for any prismatic section is simply the force divided by the cross sectional area

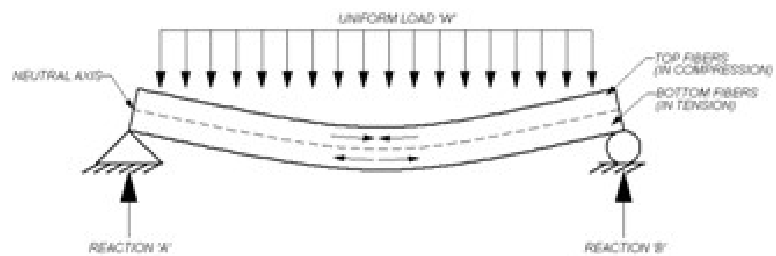


Figure 1-Beam diagram

5.2 THEORY OF SIMPLE BENDING, DERIVATION OF BENDING EQUATION

Pure Bending Stress

Bending will be called as pure bending when it occurs solely because of coupling on its end. In that case there is no chance of shear stress in the beam. But, the stress that will propagate in the beam as a result will be known as normal stress. Normal stress because it not causing any damages to beam.

Assumptions in the Theory of Bending Equation:

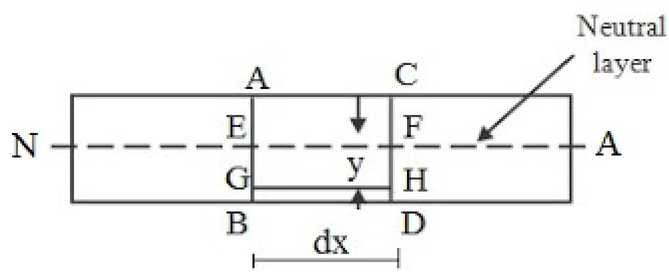
1. Material of beam is homogenous and isotropic.
2. Young's modulus is constant in compression and tension.

3. Transverse section which are plane before bending before bending remain plain after bending.
4. Beam is initially straight and all longitudinal filaments bend in circular arcs.
5. Radius of curvature is large compared with dimension of cross sections.
6. Each layer of the beam is free to expand or contract

Derivation of Bending Equation

Let us assume that following beam PQ is horizontal and supported at its two extreme ends i.e. at end P and at end Q, therefore we can say that we have considered here the condition of simply supported beam.

Now let us consider one small portion of the beam PQ, which is subjected to a simple bending, as displayed here in following figure. Let us consider two sections AB and CD as shown in following figure



Now we have following information from the above figure.

AB and CD: Two vertical sections in a portion of the considered beam

N.A: Neutral axis which is displayed in above figure

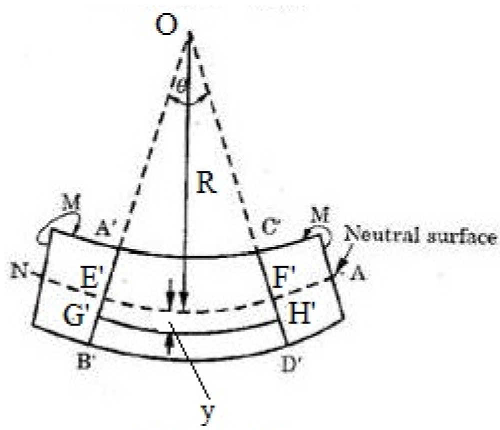
EF: Layer at neutral axis

dx = Length of the beam between sections AB and CD

Let us consider one layer GH at a distance y below the neutral layer EF. We can see here that length of the neutral layer and length of the layer GH will be equal and it will be dx.

Original length of the neutral layer EF = Original length of the layer GH = dx

Now we will analyze here the condition of assumed portion of the beam and section of the beam after bending action and we have displayed here in following figure



After bending

As we can see here that portion of the beam will be bent in the form of a curve due to bending action and hence we will have following information from above figure.

Section AB and CD will be now section A'B' and C'D'
Similarly, layer GH will be now G'H' and we can see here that length of layer GH will be increased now and it will be now G'H'
Neutral layer EF will be now E'F', but as we have discussed during studying of the various assumptions made in theory of simple bending,
Length of the neutral layer EF will not be changed.
Length of neutral layer EF = E'F' = dx
A'B' and C'D' are meeting with each other at center O as displayed in above figure

Radius of neutral layer E'F' is R as displayed in above figure
Angle made by A'B' and C'D' at center O is θ as displayed in above figure
Distance of the layer G'H' from neutral layer E'F' is y as displayed in above figure

Length of the neutral layer E'F' = R x θ
Original length of the layer GH = Length of the neutral layer EF = Length of the neutral layer E'F' = R x θ
Length of the layer G'H' = (R + y) x θ
As we have discussed above that length of the layer GH will be increased due to bending action of the beam and therefore we can write here the following equation to secure the value of change in length of the layer GH due to bending action of the beam.

Change in length of the layer GH = Length of the layer G'H'- original length of the layer GH

Change in length of the layer GH = $(R + y) \times \theta - R \times \theta$

Change in length of the layer GH = $y \times \theta$

Strain in the length of the layer GH = Change in length of the layer GH/ Original length of the layer GH

Strain in the length of the layer GH = $y \times \theta / R \times \theta$

Strain in the length of the layer GH = y/R

As we can see here that strain will be directionally proportional to the distance y i.e. distance of the layer from neutral layer or neutral axis and therefore as we will go towards bottom side layer of the beam or towards top side layer of the beam, there will be more strain in the layer of the beam.

At neutral axis, value of y will be zero and hence there will be no strain in the layer of the beam at neutral axis.

Let us recall the concept of Hook's Law

According to Hook's Law, within elastic limit, stress applied over an elastic material will be directionally proportional to the strain produced due to external loading and mathematically we can write above law as mentioned here.

$$\text{Stress} = E.\ \text{Strain}$$

$$\text{Strain} = \text{Stress} / E$$

$$\text{Strain} = \sigma / E$$

Where E is the Young's Modulus of elasticity of the material

Let us consider the above equation and putting the value of strain secure above, we will have following equation as mentioned here.

$$\sigma / E = y/R$$

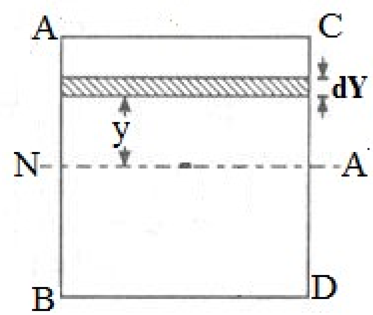
$$\sigma = (y/R) \times E$$

Therefore, bending stress on the layer will be given by following formula as displayed here

$$\sigma_b = \frac{E}{R} \times y$$

We can conclude from above equation that stress acting on layer of the beam will be directionally proportional to the distance y of the layer from the neutral axis.

Therefore, there will be force acting on the layers of the beams due to these stresses and hence there will be moment of these forces about the neutral axis too.
Total moment of these forces about the neutral axis for a section will be termed as moment of resistance of that section.
As we have already assumed that we are working here with a beam having rectangular cross-section and let us consider the cross-section of the beam as displayed here in following figure.



Let us assume one strip of thickness dy and area dA at a distance y from the neutral axis as displayed in above figure.
Let us determine the force acting on the layer due to bending stress and we will have following equation
dF = σ x dA
Let us determine the moment of this layer about the neutral axis, dM as mentioned here

dM = dF x y

dM = σ x dA x y

dM = (E/R) x y x dA x y

dM = (E/R) x y2 dA

Total moment of the forces on the section of the beam around the neutral axis, also termed as moment of resistance, could be secured by integrating the above equation and we will have

dM = (E/R) x y2 dA

$$M = \frac{E}{R} \int y2\ dA$$

$$I = \int y2\ dA$$

$$M = \frac{E}{R} x\ I$$

$$\frac{M}{I} = \frac{E}{R}$$

Let us consider the above equation of moment of resistance and equation that we have secured for bending stress in case of bending action; we will have following equation which is termed as bending equation or flexural formula of bending equation.

$$\frac{M}{I} = \frac{\sigma}{y} = \frac{E}{R}$$

5.3 USE OF THE EQUATION

Bending theory is also known as flexure theory is defined as the axial deformation of the beam due to external load that is applied perpendicularly to a longitudinal axis which finds application in applied mechanics.

5.4 CONCEPT OF MOMENT OF RESISTANCE

When a beam is subjected to bending moment, the stresses of opposite nature develop on the sections above and below the neutral axis. These internal forces have certain moments about neutral axis. The sum of moments of internal forces about neutral axis is known as moment of resistance or flexural strength.

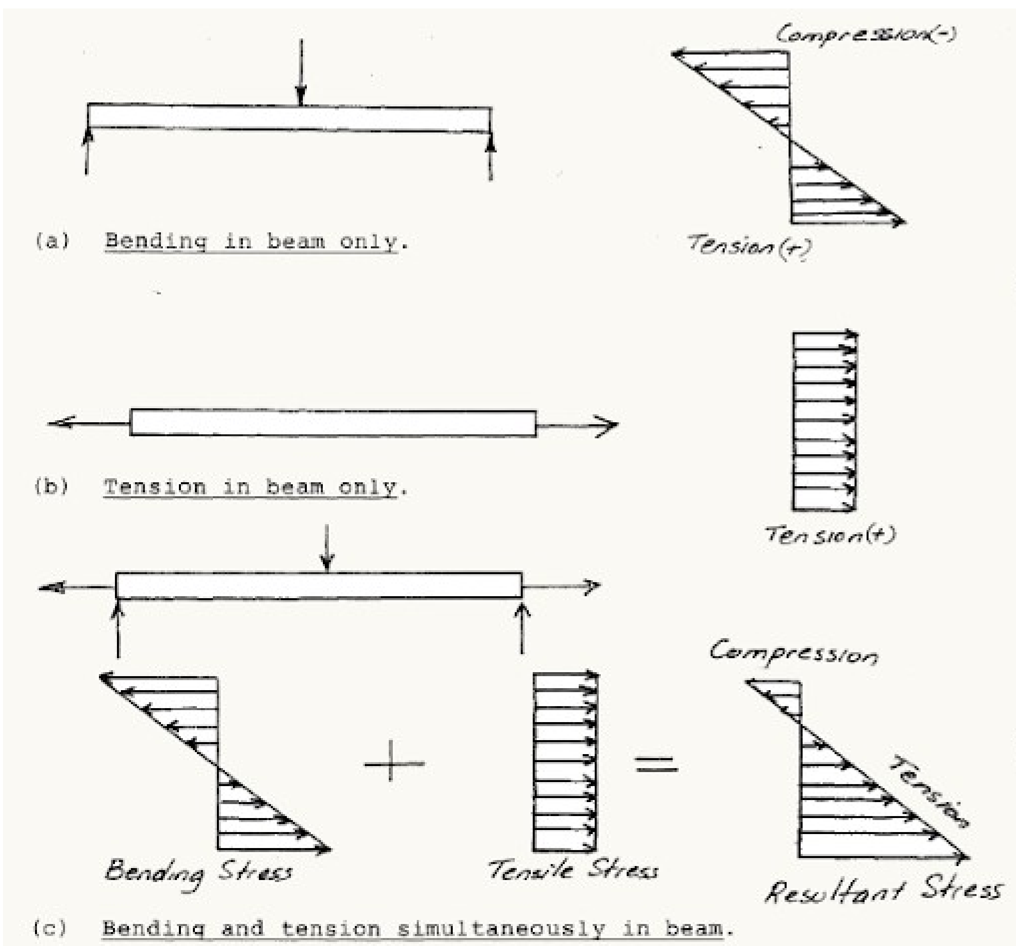
$$\frac{M}{I} = \frac{\sigma}{\mathrm{Y}}$$

$$M = \sigma \frac{\mathrm{I}}{\mathrm{Y}}$$

$$M = \sigma Z$$

Where Z is known as Section Modulus. So, moment of resistance is directly proportional to its section modulus of beam.

5.5 BENDING STRESS DIAGRAM



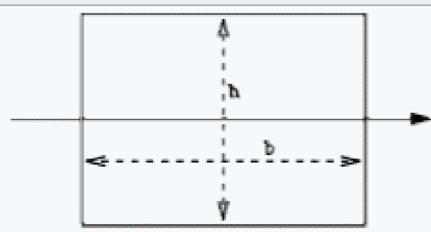
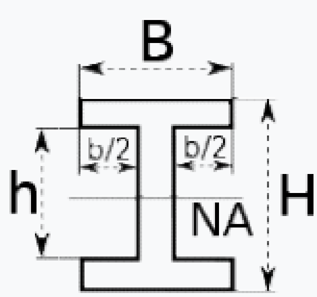
5.6 SECTION MODULUS FOR RECTANGULAR, CIRCULAR AND SYMMETRICAL I SECTION

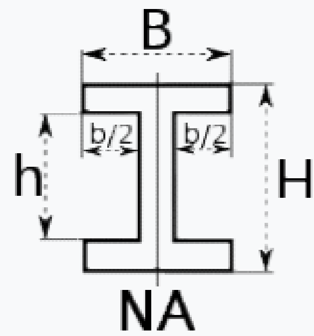
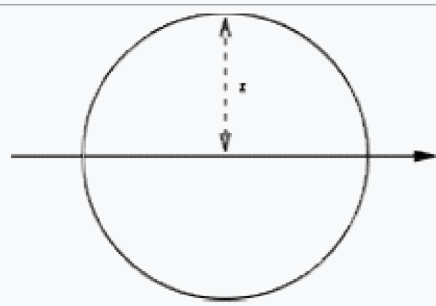
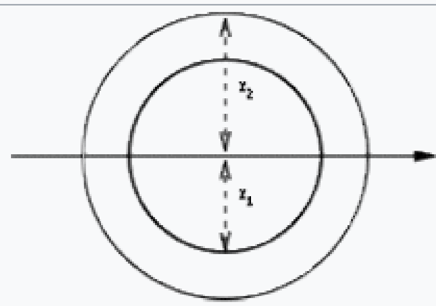
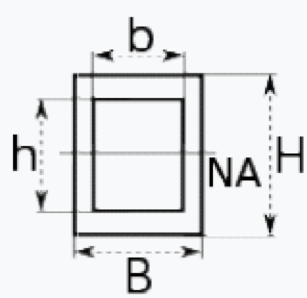
It is termed as the ratio of second moment of area and distance from N.A (Neutral axis) to the extreme fiber. Also it is the measure of strength of given member. The stress in the outermost section of beam is computed with the help of section modulus. It is indicated by S.

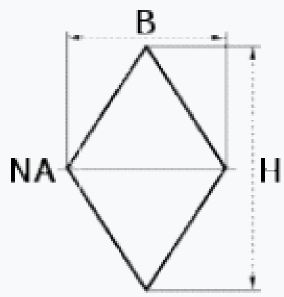
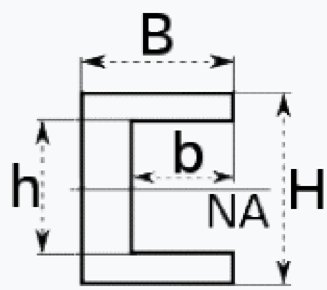
Section modulus is indicated as follows:

$$S = \frac{I}{y}$$

Here, I is "moment of inertia" and y is distance from "neutral axis" to top or bottom of fiber. Section modulus depends only on the cross section shape of the beam. Cross section shapes like rectangular, square, circular, I section and T, composite section etc.

Cross-sectional shape	Figure	Equation
Rectangle		$S = \frac{bh^2}{6}$
doubly symmetric I-section (major axis)		$Sx = \frac{BH^2}{6} - \frac{bh^3}{6H}$ $Sx = \frac{Ix}{y}$, with $y = \frac{H}{2}$

doubly symmetric **I-section** (minor axis)		$Sy = \frac{B^2(H-h)}{6} + \frac{(B-b)^3 h}{6B}$
Circle		$S = \frac{\pi d^3}{32}$
Circular hollow section		$S = \frac{\pi\left(r_2^4 - r_1^4\right)}{4r_2} = \frac{\pi(d_2^4 - d_1^4)}{32d_2}$
Rectangular hollow section		$S = \frac{BH^2}{6} - \frac{bh^3}{6H}$

Diamond		$S = \frac{BH^2}{24}$
C-channel		$S = \frac{BH^2}{6} - \frac{bh^3}{6H}$

5.7 CALCULATION OF MAXIMUM BENDING STRESS IN BEAMS OF RECTANGULAR, CIRCULAR, AND T SECTION

Maximum bending stress in Beam of Rectangular section

Example 5.3. **A simply supported beam of 5 m span and of rectangular section of 60 mm × 100 mm placed with longer leg vertical carries a uniformly distributed load of 5000 N/m over the whole span. Calculate the maximum bending stress developed in the section at**

(i) supports,

(ii) 1 m from the supports,

(iii) middle of span.

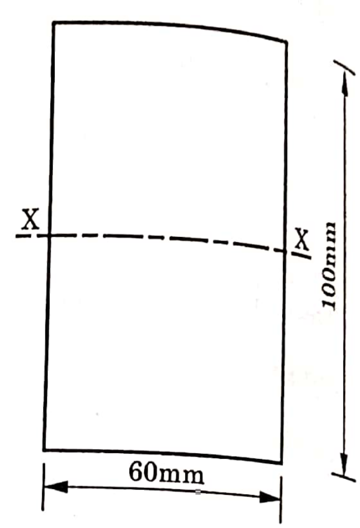


Fig. 5.12 (a)

Solution. Given, l = 5m = 5000 mm, b = 60 mm, d = 100 mm, w = 5000 N/m.

M.O.I. about horizontal centroidal axis,

$$I_{xx} = \frac{bd^3}{12} = \frac{60 \times 100^3}{12}$$

$$= 5 \times 10^6\, mm^4$$

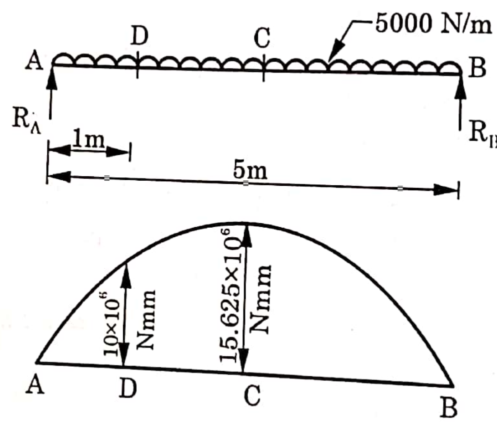


Fig. 5.12(b)

For equilibrium of beam,

$$R_A + R_B = 5{,}000 \times 5 = 25{,}000 \text{ N}$$

As the beam is symmetrically loaded, therefore reactions at both the supports will be equal.

$\therefore \quad R_A = R_B = \frac{25000}{2} = 12500$ N

B.M. at support $A = 0$

B.M. at D (*i.e.* 1 m from support A) $= 12500 \times 1 - 5000 \times 1 \times \frac{1}{2}$

$= 10{,}000$ Nm

$= 10 \times 10^6$ Nmm

B.M. at C (middle of span) $= 12500 \times \frac{5}{2} - 5000 \times \frac{5}{2} \times \frac{5}{4}$

$= 31250 - 15625 = 15625$ Nm

$= 15.625 \times 10^6$ Nmm

B.M. at support $B = 0$

Maximum bending stress (for any section) will be at the extreme fibre (*i.e.* $\frac{d}{2}$ from N.A.).

$$\therefore \quad y = \frac{100}{2} = 50 \text{ mm}$$

We know that

$$\frac{M}{I} = \frac{\sigma}{Y}$$

or

$$\sigma = \frac{M}{I} \times y$$

Bending stress at supports $= 0$ **Ans.**

Bending stress at 1m from supports, $\sigma = \frac{M}{I} y$

$$= \frac{10 \times 10^6 \times 50}{5 \times 10^6} = 100 \text{ N/mm}^2 \quad \textbf{Ans.}$$

Bending stress at mid of span $= \frac{M}{I} y$

$$= \frac{15.625 \times 10^6 \times 50}{5 \times 10^6}$$

$$= 156.25 \text{ N/mm}^2 \quad \textbf{Ans.}$$

Maximum bending stress in Beam of Circular and T section

Example 5.12. The cross section of cast iron beam is shown in the fig. 5.25. This beam is simply supported at the ends and carries a uniformly distributed load of 20 kN/m. If the span of the beam is 3m, determine the maximum tensile and compressive stresses in the beam.

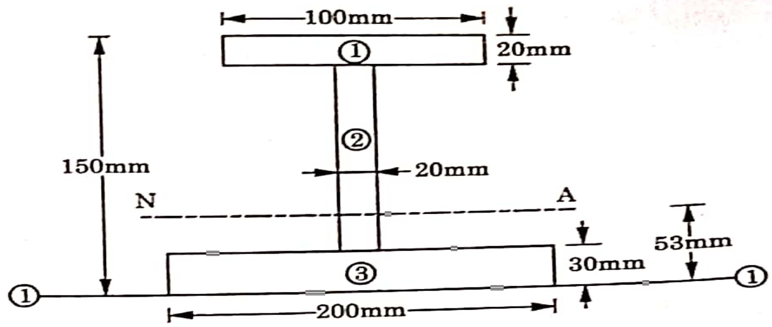


Fig. 5.25

Let us split this I- section into three components- Component (1), component (2) and component (3).

Component	Area(A) (mm^2)	Centroidal Distance y from Axis 1–1 (mm)	Ay (mm^3)	Ay^2 (mm^4)	I_{self} (mm^4)
1.	2000	140	280000	39200000	$\frac{10\times(20)^3}{12} = 66667$
2.	2000	80	160000	12800000	$\frac{20\times(100)^3}{12} = 1666667$
3.	6000	15	90000	1350000	$\frac{20\times(30)^3}{12} = 450000$
	$\Sigma A =$ 10000		$\Sigma Ay =$ 530000	$\Sigma Ay^2 =$ 53350000	$\Sigma I_{self} = 2183334$

Distance of centroidal axis XX (N.A.)from axis 1–1,

$$\bar{y} = \frac{\Sigma Ay}{\Sigma A}$$

$$= \frac{530000}{10000}$$

$$= 53 \text{ mm}$$

$$I_{1-1} = \Sigma I_{self} + \Sigma Ay^2$$

$$= 2183334 + 53350000$$

$$= 55533334 \text{ mm}^4$$

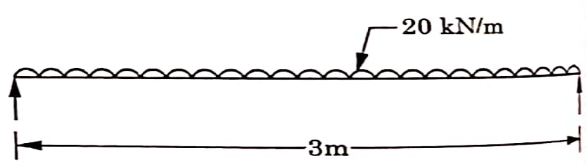


Fig. 5.26

But $\quad I_{1-1} = I_{XX} + (\Sigma A).\ \bar{y}^2$

$$55533334 = I_{XX} + 10000 \times (53)^2$$

or $$I_{XX} = 27443334 \text{ mm}^4.$$

$$\text{Maximum B.M., } M = \frac{wl^2}{8} = \frac{20 \times 10^3 \times 3 \times 3 \times 1000}{8}$$

$$= 22.5 \times 10^6 \text{ Nmm}$$

For a simply supported beam, the tensile stress will be at the extreme bottom fibre and compressive stress will be at the extreme top fibre.

Maximum tensile bending stress (at bottom fibre) :

Distance of extreme bottom fibre form N.A.,

$$y_t = 53 \text{ mm}$$

$$\sigma_t = \frac{M}{I} y_t = \frac{22.5 \times 10^6}{27443334} \times 53$$

$$= 43.453 \text{ N/mm}^2$$

Maximum compressive bending stress (at top fibre) :

Distance of extreme top fibre form N.A.,

$$y_c = 150 - 53 = 97 \text{ mm}$$

$$\therefore \quad \sigma_c = \frac{M}{I} y_c = \frac{22.5 \times 10^6}{27443334} \times 97$$

$$= 79.527 \text{ N/mm}^2$$

Hence, maximum tensile stress (bottom fibre) = 43.453 N/mm² **Ans.**

Maximum compressive stress (top fibre) = 79.527 N/mm² **Ans.**